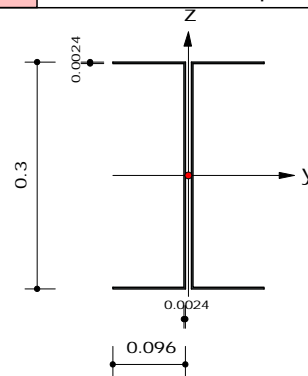
	Company		Project Title	2F-3Bx11B Site Office at Jalan Buk
	Author	Cheng Tee Teck	File Name	\\1...x12B worker quarter-rev1.mgb

1. Design Information

Design Code : BS5950-90
 Unit System : kN, m
 Member No : 11668
 Material : S275 (No:1)
 (Fy = 275000, Es = 210000000)
 Section Name : C30024 (No:2)
 (Rolled : C30024).
 Member Length : 5.46100



2. Member Forces

Axial Force Fxx = 0.53524 (LCB: 1, POS:1/2)
 Bending Moments My = 15.0634, Mz = 0.00000
 End Moments Myi = -6.2778, Myj = -2.6960 (for Le)
 Myi = -6.2778, Myj = -2.6960 (for Ly)
 Mzi = 0.00000, Mzj = 0.00000 (for Lz)
 Shear Forces Fyy = 0.00000 (LCB: 3, POS:1/2)
 Fzz = -14.976 (LCB: 1, POS:I)

Depth	0.30000	Web Thick	0.00240
Flg Width	0.09600	Flg Thick	0.00240
BTB Spacing	0.00800		
Area	0.00234	Asz	0.00144
Qyb	0.02518	Qzb	0.00461
Iyy	0.00003	Izz	0.00000
Ybar	0.10000	Zbar	0.15000
Zyy	0.00020	Zzz	0.00003
ry	0.11457	rz	0.03721

3. Design Parameters

Effective Length for LTB Le = 5.46100
 Effective Length Factors Ky = 1.00, Kz = 1.00
 Equivalent Uniform Moment Factors / Slenderness Correction Factor
 m_y = 1.00, m_z = 1.00, n = 1.00

4. Checking Results

Slenderness Ratio
 $KL/r = 146.7 < 200.0$ (Memb:11667, LCB: 8)..... O.K
 Axial Resistance
 $Ft/Pt = 0.535/173.812 = 0.003 < 1.000$ O.K
 Bending Resistance
 $My/Mcy = 15.0634/15.2097 = 0.990 < 1.000$ O.K
 $Mz/Mcz = 0.00000/2.40721 = 0.000 < 1.000$ O.K
 Combined Capacity (Tension+Bending)
 $Rmax = Fa/Pa + My/Mcy + Mz/Mcz = 1.625 > 1.000$ N.G
 Shear Resistance
 $Fvy/Pvy = 0.000 < 1.000$ O.K
 $Fvz/Pvz = 0.237 < 1.000$ O.K